

Strength of Materials

Notes by-

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Strain Energy

SOM
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Axial force : Tensile or compressive : contraction or Elongation ($\delta L, \delta b, \delta d$)
 Torque : Angular deformation (Twist) ($\delta \theta$)
 Bending moment : Angular deformation. ($\delta \theta$)

* Proof Resilience: The max. strain energy which can be stored by a body without undergoing permanent deformation. i.e. it is the strain energy at elastic limit.

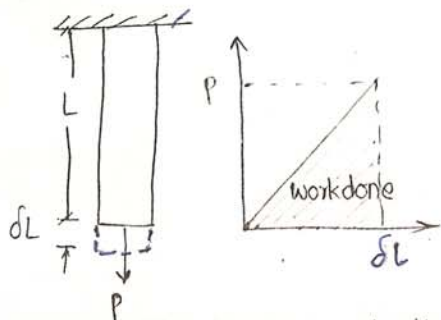
Let f_y = stress at elastic limit

$$\text{Proof Resilience} = \frac{f_y^2}{2E} \times \text{Volume}$$

* Modulus of Resilience: (Proof resilience per unit vol)

$$\text{Modulus of Resilience} = \frac{f_y^2}{2E} \quad (\text{MPa})$$

* Stress due to Gradually applied load



The load 'P' is applied gradually from zero to 'P'.

at $P=0$; $\delta L=0$
 at $P=P$; $\delta L=\delta L$

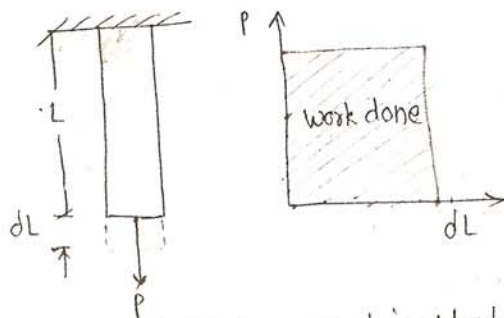
Equating ; work energy to strain energy

Work energy = strain energy

$$\frac{0+P}{2} \cdot \delta L = \frac{1}{2} \cdot P \cdot \delta L = \frac{1}{2} \cdot \delta \cdot A \cdot \delta L$$

$$\delta = \frac{P}{A}$$

* Stress due to suddenly applied load



The load 'P' is applied suddenly.
 \therefore P is constant throughout the process of extension.

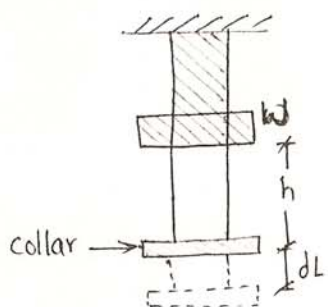
work done = strain energy

$$\frac{(P+P)}{2} \cdot \delta L = P \cdot \delta L = \frac{1}{2} \cdot \delta \cdot A \cdot \delta L$$

$$\delta = \frac{2P}{A}$$

Thus ; stress produced by suddenly applied load ; is twice that of gradually applied load.

* Stress due to Impact load



Let the load W falls freely from dist h, before it strikes the rigid collar attached at the bottom of the bar.

$$\therefore \text{work done} = W(h + \delta L)$$

$$\text{Strain energy} = \frac{1}{2} \cdot \delta \cdot A \cdot \delta L$$

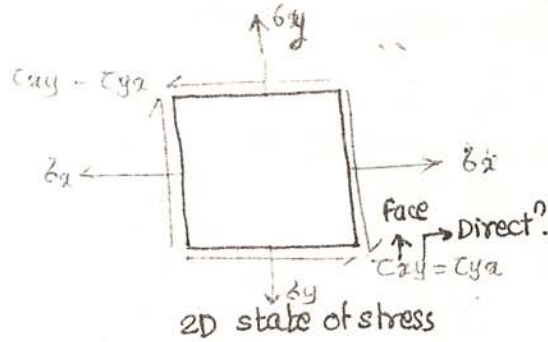
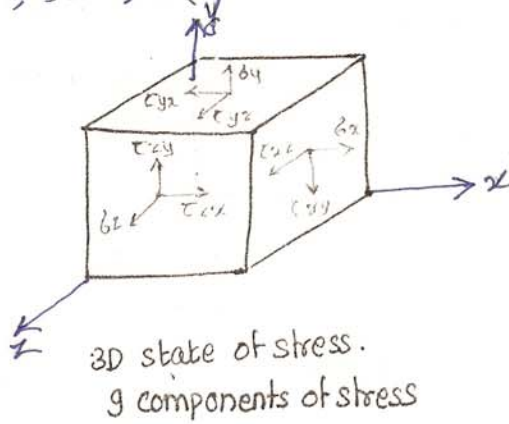
$$\therefore \delta = \frac{W}{A} \left[1 \pm \sqrt{1 + \frac{2AhE}{WL}} \right]$$

Principal stresses & strains

shear stress: Tangential to plane
 Normal stress: \perp ar (Normal) to the plane.

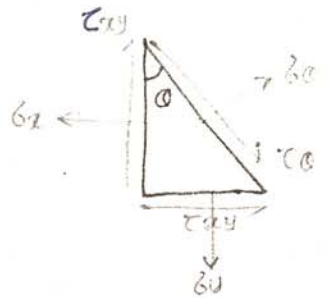
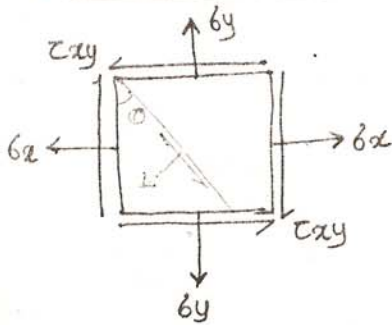
When shear stress & Normal stress acts together; they result in to oblique stresses (NOT RESULTANT STRESSES).

उत्तरा, कलता, विरपा.



P60:-
 4
 20MPa

Transformation of plane stress



$$\left. \begin{aligned} \sigma_\theta &= \left(\frac{\sigma_x + \sigma_y}{2}\right) + \left(\frac{\sigma_x - \sigma_y}{2}\right) \cos 2\theta + \tau_{xy} \sin 2\theta \\ \tau_\theta &= \left(\frac{\sigma_x - \sigma_y}{2}\right) \sin 2\theta + \tau_{xy} \cos 2\theta \end{aligned} \right\} \text{stress Transformat}^n \text{ eq}^n$$

$$\tan \phi = \frac{\tau_\theta}{\sigma_\theta} \quad R = \sqrt{\tau_\theta^2 + \sigma_\theta^2}$$

The planes on which max. or min. normal stress occurs; & there is no shear stress are known as "Principal Planes". & stress acting on these planes - max. & min. normal stress are called as "principal stresses".

\therefore To locate plane of max. & min normal stresses;

$$\boxed{\tan 2\theta_n = -\frac{2\tau_{xy}}{\sigma_x - \sigma_y}}$$

where θ_n = orientatⁿ of plane of max. normal stresses.

$$\text{Also; } \sigma_3 = \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

where σ_1 = Major principal stress
 σ_3 = Minor principal stress.

* Max. shear stress :-

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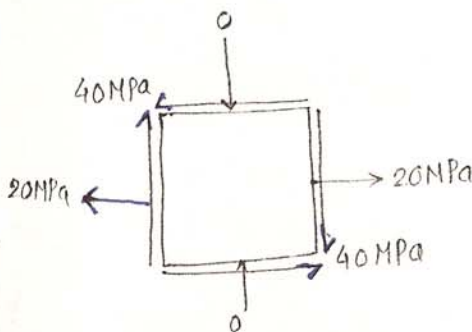
$$\tan 2\theta_s = \frac{\sigma_x - \sigma_y}{2\tau_{xy}}$$

$\theta_s =$ plane of max. shear stress

$$\tau_{\max/\min} = \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

Pro:- At a pt. in a stressed matl; the state of stress is as shown in fig. Determine :-

- ① Principal stresses
- ② Principal plane
- ③ Max. shear stress
- ④ Plane of max. shear



solⁿ:- a) Principal stresses :-

$$\sigma_{1/3} = \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

[Here; $\sigma_x = 20 \text{ MPa}$
 $\sigma_y = 0$
 $\tau_{xy} = 40 \text{ MPa}$]

$$\therefore \sigma_{1/3} = \frac{20+0}{2} \pm \sqrt{\left(\frac{20-0}{2}\right)^2 + 40^2}$$

$$\therefore \sigma_1 = 51.23 \text{ MPa (Tensile)}$$

$$\sigma_3 = -31.23 \text{ MPa (Compressive)}$$

Major principal stress
Minor principal stress

b) plane of principal stress (Principal Plane)

$$\tan 2\theta_n = \frac{-2\tau_{xy}}{\sigma_x - \sigma_y} = \frac{-2 \times 40}{20 - 0} = \tan^{-1} 4 = \frac{75.96}{2} = 37.98^\circ$$

$$\therefore \theta_n = -37.98^\circ ; -127.98^\circ \dots \text{Major principal plane}$$

$$= 90 + 37.98^\circ = 127.98^\circ$$

c) Max. shear stress :

$$\tau_{\max/\min} = \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

$$= \pm \sqrt{\left(\frac{20-0}{2}\right)^2 + 40^2}$$

$$\tau_{\max/\min} = \pm 41.23 \text{ MPa.} \dots \text{Max. \& min. shear stress.}$$

d) plane of max. shear stress.

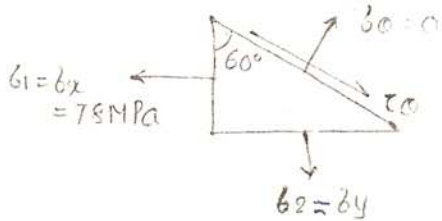
$$\tan 2\theta_s = \frac{\sigma_x - \sigma_y}{2\tau_{xy}}$$

$$= \frac{20-0}{2 \times 40}$$

$$\therefore \theta_s = 7.02^\circ ; 97.02^\circ$$

Prob: At a point in a strained mat. subjected to 2D state of stress, one of the principal stress is 78 MPa; tensile on a plane at 60° to this principal plane; the normal stress is zero.

- Determine
- (1) Other principal stress
 - (2) shear stress on the plane of zero normal stress
 - (3) ~~The plane on which normal & shear stresses are equal in magnitude~~



1] Principal stress:-

$$\sigma_\theta = \left(\frac{\sigma_x + \sigma_y}{2}\right) + \left(\frac{\sigma_x - \sigma_y}{2}\right) \cdot \cos 2\theta - \tau_{xy} \sin 2\theta$$

As it is given that principle stress $= \sigma_\theta = 0$

$$\tau_{xy} = 0$$

$$\sigma_x = 78 \text{ MPa}$$

$$\therefore 0 = \left(\frac{78 + \sigma_y}{2}\right) + \left(\frac{78 - \sigma_y}{2}\right) \cdot \cos(2 \times 60)$$

$$\therefore 39 + 39 \times \cos(120) + 0.5\sigma_y - 0.5\sigma_y \cos 120$$

$$\boxed{\sigma_y = -26.67 \text{ MPa (comp.)}}$$

2] Shear stress:-

$$\tau_\theta = \frac{\sigma_x - \sigma_y}{2} \sin 2\theta + \tau_{xy} \cos 2\theta$$

$$= \left[\frac{78 - (-26.67)}{2}\right] \times \sin 120$$

$$\boxed{\tau_\theta = 45.32 \text{ MPa (Tensile)}}$$