

Strength of Materials

Notes by-

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COLUMNS & STRUTS

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column: Vertical Compression member

Strut: Non-vertical compression member.

Long col^m: Fails by buckling

Short col^m: Fails by crushing.

Critical load: (Buckling load): - Max. axial load to which col^m does not buckle; but very small lateral load will cause buckling of col^m.

* Euler's Theory of long column -

Assumptions: ① col^m is long, i.e. direct stresses are insignificant to bending stresses.

② col^m is prismatic, homogeneous & uniform.

③ Load is axial compressive.

④ Modulus of elasticity (E) is same in Tension as well as in comp.

⑤ Plane of col^m remains plane after bending

⑥ Longitudinal fibres are free to expand or contract.

Case I] When both ends of col^m are pinned or hinged.

Case II] One end fixed; other end free

Case III] Both end fixed

Case IV] One end fixed other end hinged.

$P_E = \frac{\pi^2 EI}{L^2}$	$L_e = L$
$P_E = \frac{\pi^2 EI}{4L^2}$	$L_e = 2L$
$P_E = \frac{4\pi^2 EI}{L^2}$	$L_e = L/2$
$P_E = \frac{2\pi^2 EI}{L^2}$	$L_e = L/\sqrt{2}$

General formula: $P = \frac{\pi^2 EI}{L_e^2}$

L_e = Effective length of col^m
- Dist. betⁿ pt. of contraflexure.

* Limitation of Euler's formula:-

Let $P_E = \frac{\pi^2 EI}{L_e^2}$

$\therefore P_E = \frac{\pi^2 E (A \cdot r^2)}{L_e^2}$

$\therefore \frac{P_E}{A} = \frac{\pi^2 E}{\left(\frac{L_e}{r}\right)^2}$

$\therefore F_{cc} = \frac{\pi^2 E}{\lambda^2}$

$I = A \cdot r^2$
 $r = \sqrt{I/A}$

A = c/s area
r = Radius of gyration.

F_{cc} = Elastic critical stress
i.e. stress corresponding to buckling load P_E
 λ = slenderness ratio
 $= \frac{L_e}{r}$

But $F_{cc} \neq F_y$

i.e. Elastic critical stress \neq Yield stress of col^m matl.

\therefore If λ is very less, F_{cc} will be more than F_y .

\therefore For Euler's formula holds good; $\left[\lambda \geq \sqrt{\frac{\pi^2 E}{F_y}} \right]$ > Limitation
(Only for Long col^m)

3000

$\frac{200}{EI} =$

18
2546

* Rankine's formula:

Used for long as well as short col^m.

$$\frac{1}{P_R} = \frac{1}{P_C} + \frac{1}{P_E}$$

Where P_R = Rankine's Load

P_C = Crushing load = $F_y \cdot A$

P_E = Euler's load. /critical/ buckling load.

$$\therefore P_R = \frac{P_C \cdot P_E}{P_C + P_E} = \frac{P_C}{1 + \frac{P_E}{P_C}} = \frac{F_y \cdot A}{1 + \frac{F_y \cdot A}{P_E}}$$

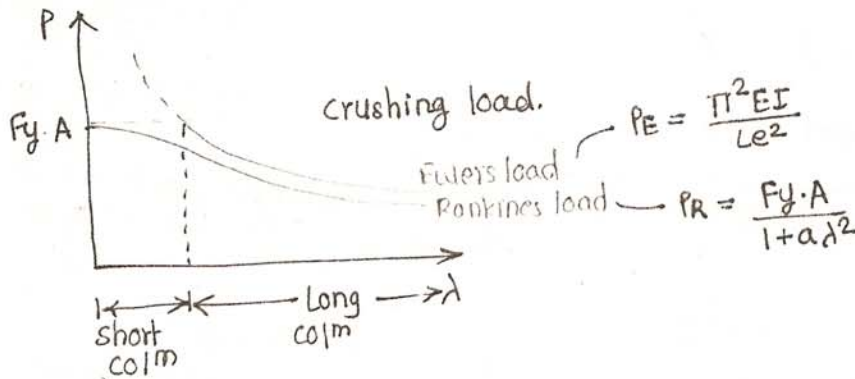
$$= \frac{F_y \cdot A}{1 + \left(\frac{F_y \cdot A}{\pi^2 EI / L^2} \right)} = \frac{F_y \cdot A}{1 + \left(\frac{F_y \cdot A \cdot L^2}{\pi^2 E I^2} \right)}$$

$$\frac{I}{r^2} = \lambda$$

$$= \frac{F_y \cdot A}{1 + \frac{F_y \cdot \lambda^2}{\pi^2 E}}$$

$$P_R = \frac{F_y \cdot A}{1 + a \lambda^2}$$

Where $a = \frac{F_y}{\pi^2 E}$ = Rankine's constant.



Pro:- A MS tube 22mm ϕ , 3mm thk, 2m long used as a strut; hinged at two ends. calculate crippling load by Eulers formula. $E = 200 \text{ GPa}$

Solⁿ:- $P_E = \frac{\pi^2 EI}{L_e^2}$

For both ends hinged; $L_e = L$

$\therefore P_E = \frac{\pi^2 EI}{L^2}$

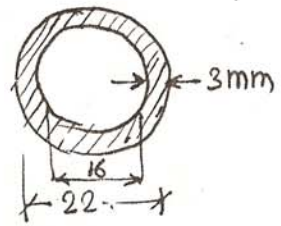
$\therefore I = \frac{\pi}{64} [D^4 - d^4] = \frac{\pi}{64} [22^4 - 16^4] = 8282 \text{ mm}^4$

$E = 200 \times 10^3 \text{ N/mm}^2$

$\therefore P_E = \frac{\pi^2 \times 200 \times 10^3 \times 8282}{(2000)^2}$

$= 4087 \text{ N}$

$P_E = 4.087 \text{ kN}$



* Find crushing load by Rankines formula for a hollow CI col^m of 200mm ext. dia. & 25mm thk. of metal. If length of col^m = 8m; both ends fixed.

$F_y = 550 \text{ MPa}, \alpha = \frac{1}{1600}$

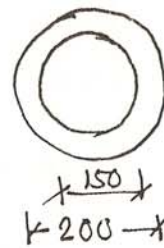
$P_R = \frac{F_y \cdot A}{1 + \alpha \lambda^2}$

$A = \frac{\pi}{4} (200^2 - 150^2) = 13.74 \times 10^3 \text{ mm}^2$

$I = \frac{\pi}{64} (200^4 - 150^4) = 53.69 \times 10^6 \text{ mm}^4$

$\lambda = \frac{L_e}{r}; \frac{L_e}{\sqrt{I/A}} = \frac{0.5 \times 8 \times 10^3}{\sqrt{\frac{53.69 \times 10^6}{13.74 \times 10^3}}} = 63.98$

$\therefore P_R = \frac{550 \times 13.74 \times 10^3}{1 + \frac{1}{1600} \times 63.98^2} = 72 \underline{2123.7 \text{ kN}}$



Pro:- A square col^m of 100x100mm c/s has a concentric longitudinal hole of 50mm dia. The length of col^m is 5m; one end fixed & other end hinged. Determine the Eulers buckling load assuming $E = 200 \text{ GPa}$

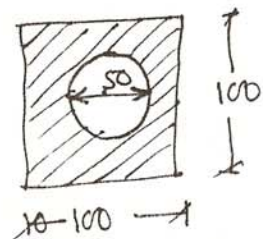
Solⁿ:- $P_E = \frac{\pi^2 IE}{L_e^2} \quad L_e = \frac{L}{\sqrt{2}}$

$I = \frac{100 \times 100^3}{12} - \frac{\pi}{64} (50)^4 = 8026.54 \text{ mm}^4$

$A = \frac{\pi}{4} 100 \times 100 - \frac{\pi}{4} (50)^2 = 8036.5 \text{ mm}^2$

$\therefore P_E = \frac{\pi^2 \times 8026.54 \times 200}{(5000/\sqrt{2})^2}$

$P_E = 1267.5 \text{ kN}$



Prob: The c/s of colⁿ is hollow rectangular section having outside dimensions $200 \times 120 \text{ mm}$ & inside dimension $180 \times 100 \text{ mm}$ having uniform thk. 10 mm . It is fixed at one end & hinged at other end. If the buckling load given by 800 kN (Rankine formula) Find actual length of colⁿ. $f_y = 200 \text{ MPa}$, $\sigma_c = 200 \text{ GPa}$, $a = 1/7500$.

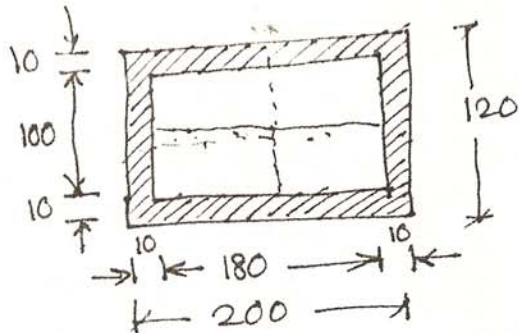
$$P_R = \frac{f_y \cdot A}{1 + a\lambda^2}$$

$$\lambda = \frac{L_e}{r}$$

$$r = \sqrt{I/A}$$

$$L_e = \frac{L}{\sqrt{2}} \therefore$$

$$\lambda = \frac{L}{r\sqrt{2}}$$



$$I = \frac{200 \times 120^3}{12} - \frac{180 \times 100^3}{12}$$

$$= 13.8 \times 10^6 \text{ mm}^4$$

$$A = 200 \times 120 - 180 \times 100 = 6 \times 10^3 \text{ mm}^2$$

$$\therefore r = 47.96 \text{ mm}$$

$$P_R = \frac{f_y \cdot A}{1 + a\lambda^2} \Rightarrow 800 \times 10^3 = \frac{200 \times 6 \times 10^3}{1 + \frac{\lambda^2}{7500}}$$

$$\therefore 1 + 1.333 \times 10^{-6} \lambda^2 = \frac{200 \times 6 \times 10^3}{800 \times 10^3} = 2.25$$

$$\therefore \lambda = 96.82$$

$$\therefore 96.82 = \frac{L}{r\sqrt{2}} \Rightarrow L = 96.82 \times 47.96 \times \sqrt{2}$$

$$\therefore L = 6.5672 \text{ m} \quad \checkmark \text{ good}$$